

Traffic Analysis Data as presented in the Project Report dated September 2008

Traffic

The purpose of the following traffic analysis was intended to first of all demonstrate operational deficiencies Perris Boulevard will experience should there be no improvement or widening implemented to handle near-term and future traffic demands. Traffic operational analysis was performed by utilizing methodology provided in accordance to the 2000 Highway Capacity Manual (HCM). The Highway Capacity Software 2000 (HCS2000) by McTrans Center was utilized for the analysis and calculations of signalized and unsignalized intersections.

Secondly, traffic study and analysis also assist in determining the number of travel lanes needed in the future. Three improvement scenarios were analyzed for this purpose involving; (1) No-Build option (two lane), (2) four-lane, and (3) six lane configurations. Operational analysis of the signalized intersections was also performed respectively for the aforementioned scenarios at Perris Boulevard and Ironwood Avenue, Kalmia Avenue, Manzanita Avenue and Pico Vista Way which has been planned for signalization in the near future, as part of this project. The City’s Transportation Engineering Division uses Average Daily Traffic (ADT) thresholds as shown below to evaluate Level-of-Service (LOS) along Perris Boulevard in-between Ironwood, Kalmia, Pico Vista Way, and Manzanita Avenue.

Table 1.0, City of Moreno Valley’s Average Daily Traffic (ADT) Threshold

Type of Roadway	Arterial Level of Service (LOS)				
	A	B	C	D	E
Two-lane divided (Existing)	11,250	13,125	15,000	16,875	18,750
Four-lane divided (Preferred)	22,500	26,300	30,000	33,800	37,500
Six-lane divided	33,900	39,400	45,000	50,600	56,300

Presently, Perris Boulevard is striped for one lane of traffic in each direction throughout much of its alignment with a double yellow or two-way left turn lane in the middle as space permits. At its intersection with Ironwood Avenue, the road widens and is able to accommodate two travel lanes northbound and southbound with a dedicated left-turn pocket for southbound traffic.

Existing and projected traffic volumes were furnished by Urban Crossroads for 2011 (Near Term), 2030 (Long Term), and 2050 (Build-Out) years. These traffic volumes were used as inputs to determine existing and projected level of service conditions along Perris Boulevard and at the intersections between Ironwood and Manzanita Avenue.

Table 1.1 summarizes the operational levels-of-service associated with the No-Build/two-lane scenario. In this scenario, Perris Boulevard is assumed to remain as a two-lane highway facility within the project limits. As shown, Average Daily Traffic along Perris Boulevard is expected to increase by 2011 and cause a failing Level-of-Service (LOS) “E” by 2030 and LOS “F” by the Build Out Year of 2050 relative to the Average Daily Traffic (ADT) thresholds established by the City’s Transportation Engineering Division. This outcome validates the need to improve and widen Perris Boulevard and increase its capacity to handle future traffic demands.

Perris Blvd Segment	Existing		Year 2011 No Build		Year 2030 No Build		Year 2050 No Build	
	ADT	LOS	ADT	LOS	ADT	LOS	ADT	LOS
Ironwood to Kalmia	17,674	E	18,137	E	20,336	F	22,514	F
Kalmia to Pico Vista	15,650	D	16,142	D	18,479	E	20,792	F
Pico Vista to Manzanita	17,868	E	18,251	E	20,072	F	21,876	F

Note:
 ADT = Average Daily Traffic
 LOS = Level of Service

Next, Perris Boulevard widening scenarios to a four and six-lane facilities were evaluated. **Table 1.2** summarizes results from applying the four-lane scenario while **Table 1.3** summarizes operational results from applying the six-lane scenario.

Segment	Existing		Yr. 2011 4 Lanes Option		Yr. 2030 4 Lanes Option		Yr. 2050 4 Lanes Option	
	ADT	LOS	ADT	LOS	ADT	LOS	ADT	LOS
Kalm to Iron	17,674	A	18,137	A	20,336	A	22,514	B
Pico to Kalm	15,650	A	16,142	A	18,479	A	20,792	A
Manz to Pico	17,868	A	18,251	A	20,072	A	21,876	A

Note:
 ADT = Average Daily Traffic
 LOS = Level of Service
Street Legend: Iron = Ironwood,, Kalm = Kalmia, Pico = Pico Vista, Manz = Manzanita

As shown in Table 1.2, widening Perris Boulevard to a minimum of two-lanes for each direction of traffic improves near and future levels of service to LOS “B” or better by 2030 and the build-out year of 2050.

Segment	Existing		Yr. 2011 6 Lanes Option		Yr. 2030 6 Lanes Option		Yr. 2050 6 Lanes Option	
	ADT	LOS	ADT	LOS	ADT	LOS	ADT	LOS
Kalm to Iron	17,674	A	18,137	A	20,336	A	22,514	A
Pico to Kalm	15,650	A	16,142	A	18,479	A	20,792	A
Manz to Pico	17,868	A	18,251	A	20,072	A	21,876	A

Note:
 ADT = Average Daily Traffic
 LOS = Level of Service
Street Legend: Iron = Ironwood,, Kalm = Kalmia, Pico = Pico Vista, Manz = Manzanita

It should be noted that if Perris Boulevard were to be widened to six lanes, striping transitions are needed at the north and south ends of the project limits. Currently, Perris Boulevard has a four-lane configuration between the its intersection with Manzanita Avenue and the joining point of the proposed improvements.

The City’s Transportation Engineering Division accepts an LOS C or better for corridor capacity analysis according to its Circulation Element of the General Plan. Tables 1.1 through 1.3 clearly reveal the ability of Perris Boulevard to handle existing and future traffic demands as a four-lane arterial based on this criteria. Therefore, improving this segment of Perris Boulevard to a six-lane facility as currently specified in the City’s Standard Plan is not justifiable.

In addition to capacity evaluations along Perris Boulevard, operational analysis at existing and future signalized intersections was also performed. The same three planning scenarios were applied, except at Perris Boulevard/Ironwood and Perris Boulevard/Manzanita Avenue intersections where a four-lane scenario is the existing configuration. The locations involved intersections along Perris Boulevard with Ironwood, Kalmia Avenue, Pico Vista Way, and Manzanita Avenue. Results of the analysis are summarized in the following **Tables 1.4A** through **1.4D**. Operational levels of service (LOS) at the intersections were analyzed with methodology consistent with the Highway Capacity Manual (HCM) 2000 edition and the HCS2000 software using turning count volumes compiled and provided by Urban Crossroads.

Tables 1.4A and **1.4B** compare traffic operational results between the existing, four-lane configuration at Perris Boulevard/Ironwood Avenue and Perris Boulevard/Manzanita Avenue versus six lane configuration at the intersections. LOS do not improve by much

between the four and six-lane scenarios. The intersections’ overall delays during PM peak hours, however, are significantly reduced. They are projected to decrease from 81 seconds to 53 seconds at Ironwood Avenue and from 105 seconds to 47 seconds at Manzanita Avenue by Year 2030.

Table 1.4A Level of Service Summary Ironwood Avenue Intersection					Table 1.4B Level of Service Summary Manzanita Avenue Intersection				
Condition	AM		PM		Condition	AM		PM	
	Int. Delay	LOS	Int. Delay	LOS		Int. Delay	LOS	Int. Delay	LOS
Four-Lane (Existing at Intersection)					Four-Lane (Existing at Intersection)				
Existing	49.6	D	52.5	D	Existing	42.3	D	47.1	D
Yr 2011	50.4	D	55	D	Yr 2011	42.8	D	51.1	D
Yr 2030	56.5	E	81.4	F	Yr 2030	47.8	D	104.5	F
Yr 2050	68.6	E	128.2	F	Yr 2050	77.5	E	193.6	F
With 6 Lanes on Perris Blvd.-Alt. Scenario					With 6 Lanes on Perris Blvd- Alt. Scenario				
Yr 2011	45	D	47.4	D	Yr 2011	39.7	D	40.6	D
Yr 2030	47	D	53.1	D	Yr 2030	41.1	D	47	D
Yr 2050	50.4	D	62.2	E	Yr 2050	43.8	D	83.3	F
Note: Int. Delay = Intersection Delay in seconds Yr = Year					Note: Int. Delay = Intersection Delay in seconds Yr = Year				

The City’s Transportation Engineering Division accepts a LOS “D” or better for operational analysis at signalized or unsignalized intersections. Based on this criteria, the two intersections will be operating at an unacceptable LOS through the year 2030 and 2050 during PM peak hour. A follow-up analysis indicates that the intersections’ level of service can be improved from LOS “E” to “D” at Perris Boulevard and Ironwood Avenue when an additional left turn lane is introduced for the northbound-to-westbound traffic movement. Similarly, the operational level of service at Perris Boulevard and Manzanita Avenue can be improved from LOS “F” to “D” when an east-to-northbound left turn lane is added to the west leg of the Manzanita Avenue intersection coupled with the introduction of an exclusive southbound right turn lane on Perris Boulevard. The necessity for these suggested improvements needs to be reevaluated in the year 2030 and 2050 respectively based on actual traffic demands.

Table 1.4C summarizes a similar analysis for the intersection of Perris Boulevard and Kalmia Avenue. At this intersection, the analysis includes an exclusive left-turn pocket option for southbound left-turning traffic on Kalmia Avenue. The results reveal the need for this exclusive left-turn pocket. Without the exclusive left-turn pocket, a failing LOS “F” is expected for the intersection during AM peak hours. The addition the exclusive left-turn pocket on Kalmia is recommended through restriping the existing pavement

area. The suggested improvement will help reduce intersection delays to stay between 20 to 26 seconds versus 36 to 41 seconds for any of the horizon years, resulting in an acceptable LOS “C”.

Table 1.4C Level of Service Summary Kalmia Ave. Intersection								
Condition	Without Turn Pocket on Kalmia				With Turn Pocket on Kalmia			
	AM		PM		AM		PM	
	Int. Delay	LOS	Int. Delay	LOS	Int. Delay	LOS	Int. Delay	LOS
Existing Condition (Two-Lane Facility at Perris Blvd/Kalmia Ave)								
Existing	187	F	44.2	D	187	F	44.2	D
Yr 2011	190.6	F	47.5	D	190.6	F	47.5	D
Yr 2030	201.1	F	85.2	F	201.1	F	85.2	F
Yr 2050	210.5	F	156.4	F	210.5	F	156.4	F
4 Lanes on Perris Blvd.								
Yr 2011	168.1	F	37.6	D	25.4	C	21.6	C
Yr 2030	172.3	F	40	D	25.4	C	23.5	C
Yr 2050	175.5	F	44.9	D	25.5	C	26.5	C
6 Lanes on Perris Blvd.								
Yr 2011	166.5	F	36.3	D	23.8	C	20.2	C
Yr 2030	170.5	F	37.7	D	23.6	C	21.2	C
Yr 2050	173.4	F	40.7	D	23.4	C	22.4	C
Note: Int. Delay = Intersection Delay in seconds. Yr = Year								

Table 1.4D summarizes the intersection operation analysis of Pico Vista Way and Perris Boulevard which is currently unsignalized. LOS of an unsignalized intersection is determined by the amount of delay experienced on the minor approach which in this case is the westbound traffic flow coming from Pico Vista Way. Based on volume, signalizing the intersection is not warranted. The City’s Transportation Engineering division recommends signalization, however, for safety reasons. As shown in Attachment A, Pico Vista Way joins the curvilinear portion of Perris Boulevard at a skew. This condition significantly limits intersection’s sight distance and restricts motorists’ ability to see oncoming traffic from any direction as they approach Perris Boulevard on Pico Vista Way.

Results of the analysis are as shown below. As expected, signalization of the intersection increases the amount of delay for individual vehicles as represented by the “worsening” LOS.

Table 1.4D Level of Service Summary Pico Vista Way Intersection				
Condition	AM		PM	
	App. Delay	LOS	App. Delay	LOS
No Build Option (Exist. Perris Blvd as a two-lane highway facility)				
Existing	8.3	A	7.9	A
Yr 2011	8.4	A	8.1	A
Yr 2030	8.6	A	9.1	A
Yr 2050	8.9	A	12.7	B
4 Lanes on Perris Blvd. (Signalized)				
Yr 2011	22.2	C	19.4	B
Yr 2030	22.3	C	21.7	C
Yr 2050	22.4	C	30.4	C
6 Lanes on Perris Blvd. (Signalized)				
Condition	Int. Delay	LOS	Int. Delay	LOS
Yr 2011	20.3	C	17.8	B
Yr 2030	20.2	C	19.2	B
Yr 2050	20.1	C	22.9	C
Note: App. Delay = Approach Delay in seconds. Int. Delay = Intersection Delay in seconds. Yr = Year				

On another note, offset distances between Pico Vista Way, Fran Lou Drive and Robin Lane do not meet the 150-foot minimum spacing criteria established by AASHTO, a nationwide authority in roadway geometric design guidelines. Therefore, they pose serious safety concerns. These concerns can be mitigated, however, by constructing a raised median along the curvilinear portion of Perris Boulevard where these intersections are currently located.

ATTACHMENT G

Traffic Survey and Analysis

Existing and Forecasted Volumes

TABLE 1

DAILY TRAFFIC VOLUMES WITH PROJECT

ROADWAY	FROM	TO	VOLUME						% GROWTH	
			COUNT (NB/EB)	COUNT (SB/WB)	COUNT (TOTAL)	BUILDOUT (NB/EB)	BUILDOUT (SB/WB)	BUILDOUT (TOTAL)		
Perris Bl.	Manzanita Av.	n/o Manzanita Av.	7,275	7,286	14,561	12,341	12,360	24,701	10,140	70%
Perris Bl.	Manzanita Av.	Pico Vista Wy.	8,810	8,864	17,674	11,223	11,291	22,514	4,840	27%
Perris Bl.	Pico Vista Wy.	Kalmia Av.	7,125	8,525	15,650	9,466	11,326	20,792	5,142	33%
Perris Bl.	Kalmia Av.	Ironwood Av.	8,976	8,892	17,868	10,989	10,887	21,876	4,008	22%
Perris Bl.	Ironwood Av.	s/o Ironwood Av.	13,184	11,848	25,032	16,091	14,461	30,552	5,520	22%
Manzanita Av.	Perris Bl.	e/o Perris Bl.	431	500	931	905	1,050	1,955	1,024	110%
Manzanita Av.	Perris Bl.	w/o Perris Bl.	1,644	1,656	3,300	3,687	3,713	7,400	4,100	124%
Pico Vista Wy.	Perris Bl.	e/o Perris Bl.	56	200	256	62	220	282	26	10%
Kalmia Av.	Perris Bl.	e/o Perris Bl.	1,904	1,852	3,756	2,094	2,037	4,132	376	10%
Ironwood Av.	Perris Bl.	e/o Perris Bl.	6,306	6,494	12,800	6,937	7,143	14,080	1,280	10%
Ironwood Av.	Perris Bl.	w/o Perris Bl.	6,219	6,844	13,063	6,841	7,528	14,369	1,306	10%

TABLE 3

TRUCK PERCENT (OVERALL)

ROADWAY	FROM	TO	PERCENT				
			AUTO	2 AXLE	3 AXLE	4+ AXLE	TOTAL
Perris Bl.	Manzanita Av.	n/o Manzanita Av.	98.2	1.6	0.1	0.1	100.0
Perris Bl.	Manzanita Av.	Pico Vista Wy.	96.2	3.2	0.2	0.4	100.0
Perris Bl.	Pico Vista Wy.	Kalmia Av.	96.2	3.2	0.2	0.4	100.0
Perris Bl.	Kalmia Av.	Ironwood Av.	96.3	3.2	0.2	0.3	100.0
Perris Bl.	Ironwood Av.	s/o Ironwood Av.	98.1	1.4	0.2	0.3	100.0
Manzanita Av.	Perris Bl.	e/o Perris Bl.	97.4	2.2	0.3	0.1	100.0
Manzanita Av.	Perris Bl.	w/o Perris Bl.	97.4	2.2	0.3	0.1	100.0
Pico Vista Wy.	Perris Bl.	e/o Perris Bl.	97.4	2.2	0.3	0.1	100.0
Kalmia Av.	Perris Bl.	e/o Perris Bl.	97.4	2.2	0.3	0.1	100.0
Ironwood Av.	Perris Bl.	e/o Perris Bl.	97.4	2.2	0.3	0.1	100.0
Ironwood Av.	Perris Bl.	w/o Perris Bl.	97.4	2.2	0.3	0.1	100.0

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TABLE 5

NIGHTTIME (7PM-7AM) TRUCK PERCENT

ROADWAY	FROM	TO	PERCENT				
			AUTO	2 AXLE	3 AXLE	4+ AXLE	TOTAL
Perris Bl.	Manzanita Av.	n/o Manzanita Av.	98.3	1.5	0.1	0.1	100.0
Perris Bl.	Manzanita Av.	Pico Vista Wy.	96.7	2.7	0.3	0.3	100.0
Perris Bl.	Pico Vista Wy.	Kalmia Av.	96.7	2.7	0.3	0.3	100.0
Perris Bl.	Kalmia Av.	Ironwood Av.	96.8	2.6	0.3	0.3	100.0
Perris Bl.	Ironwood Av.	s/o Ironwood Av.	98.3	1.2	0.1	0.4	100.0
Manzanita Av.	Perris Bl.	e/o Perris Bl.	97.2	2.4	0.3	0.1	100.0
Manzanita Av.	Perris Bl.	w/o Perris Bl.	97.2	2.4	0.3	0.1	100.0
Pico Vista Wy.	Perris Bl.	e/o Perris Bl.	97.2	2.4	0.3	0.1	100.0
Kalmia Av.	Perris Bl.	e/o Perris Bl.	97.2	2.4	0.3	0.1	100.0
Ironwood Av.	Perris Bl.	e/o Perris Bl.	97.2	2.4	0.3	0.1	100.0
Ironwood Av.	Perris Bl.	w/o Perris Bl.	97.2	2.4	0.3	0.1	100.0

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TABLE 6

INTERSECTION FORECAST GROWTH

APPROACH	MOVEMENT	EXISTING COUNT		BUILDOUT FORECAST		GROWTH		%GROWTH	
		AM	PM	AM	PM	AM	PM	AM	PM
Perris Bl. (NS) & Ironwood Av. (EW)									
Northbound	Left	173	256	248	330	75	74	43%	29%
	Through	495	483	544	849	49	366	10%	76%
	Right	160	172	176	189	16	17	10%	10%
Southbound	Left	113	136	124	149	11	13	10%	10%
	Through	519	662	709	987	190	325	37%	49%
	Right	135	72	154	80	19	8	14%	11%
Eastbound	Left	88	69	97	100	9	31	10%	45%
	Through	220	208	242	366	22	158	10%	76%
	Right	174	236	289	345	115	109	66%	46%
Westbound	Left	185	140	203	154	18	14	10%	10%
	Through	284	175	332	243	48	68	17%	39%
	Right	150	105	165	143	15	38	10%	36%

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TABLE 7

INTERSECTION FORECASTS (INTERIM YEARS)

APPROACH	MOVEMENT	EXISTING COUNT		2011		2030		BUILDOUT FORECAST	
		AM	PM	AM	PM	AM	PM	AM	PM
Perris Bl. (NS) & Ironwood Av. (EW)									
Northbound	Left	173	256	180	263	214	297	248	330
	Through	495	483	500	518	522	684	544	849
	Right	160	172	162	174	169	181	176	189
Southbound	Left	113	136	114	137	119	143	124	149
	Through	519	662	537	693	624	841	709	987
	Right	135	72	137	73	145	76	154	80
Eastbound	Left	88	69	89	72	93	86	97	100
	Through	220	208	222	223	232	295	242	366
	Right	174	236	185	246	237	296	289	345
Westbound	Left	185	140	187	141	195	148	203	154
	Through	284	175	289	181	310	212	332	243
	Right	150	105	151	109	158	126	165	143

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Perris Boulevard/Kalmia Avenue
Intersection LOS Analysis

CAPACITY AND LOS WORKSHEET										
General Information										
Project Description <i>Perris & Kalmia (BO 2050 PM) - [6 lanes improvement]</i>										
Capacity Analysis										
	EB		WB		NB			SB		
Lane group		TR		LTR		LT	R	L	TR	
Adj. flow rate		0		99		976	154	66	1124	
Satflow rate		1863		176		5085	1583	1770	5085	
Lost time		2.0		2.0		2.0	2.0	2.0	2.0	
Green ratio		0.30		0.30		0.35	0.35	0.22	0.60	
Lane group cap.		567		54		1769	551	385	3051	
v/c ratio		0.00		1.83		0.55	0.28	0.17	0.37	
Flow ratio		0.00		0.56		0.19	0.10	0.04	0.22	
Crit. lane group		N		Y		Y	N	Y	N	
Sum flow ratios	0.79									
Lost time/cycle	15.00									
Critical v/c ratio	0.91									
Lane Group Capacity, Control Delay, and LOS Determination										
	EB		WB		NB			SB		
Lane group		TR		LTR		LT	R	L	TR	
Adj. flow rate		0		99		976	154	66	1124	
Lane group cap.		567		54		1769	551	385	3051	
v/c ratio		0.00		1.83		0.55	0.28	0.17	0.37	
Green ratio		0.30		0.30		0.35	0.35	0.22	0.60	
Unif. delay d1		27.8		40.0		30.3	27.1	36.6	11.8	
Delay factor k		0.50		0.50		0.50	0.50	0.50	0.50	
Increm. delay d2		0.0		437.8		1.2	1.3	1.0	0.3	
PF factor		1.000		1.000		1.000	1.000	1.000	1.000	
Control delay		27.8		477.8		31.5	28.4	37.5	12.2	
Lane group LOS		C		F		C	C	D	B	
Apprch. delay			477.8		31.1			13.6		
Approach LOS			F		C			B		
Intersec. delay	40.7		Intersection LOS					D		

CAPACITY AND LOS WORKSHEET										
General Information										
Project Description <i>Perris & Kalmia (IY 2030 PM) - [6 lanes improvement]</i>										
Capacity Analysis										
	EB		WB		NB		SB			
Lane group	TR		LTR		LT	R	L	TR		
Adj. flow rate	0		94		767	147	62	960		
Satflow rate	1863		191		5085	1583	1770	5085		
Lost time	2.0		2.0		2.0	2.0	2.0	2.0		
Green ratio	0.30		0.30		0.35	0.35	0.22	0.60		
Lane group cap.	567		58		1769	551	385	3051		
v/c ratio	0.00		1.62		0.43	0.27	0.16	0.31		
Flow ratio	0.00		0.49		0.15	0.09	0.04	0.19		
Crit. lane group	N		Y		N	N	N	Y		
Sum flow ratios	0.68									
Lost time/cycle	11.00									
Critical v/c ratio	0.75									
Lane Group Capacity, Control Delay, and LOS Determination										
	EB		WB		NB		SB			
Lane group	TR		LTR		LT	R	L	TR		
Adj. flow rate	0		94		767	147	62	960		
Lane group cap.	567		58		1769	551	385	3051		
v/c ratio	0.00		1.62		0.43	0.27	0.16	0.31		
Green ratio	0.30		0.30		0.35	0.35	0.22	0.60		
Unif. delay d1	27.8		40.0		28.8	27.0	36.5	11.3		
Delay factor k	0.50		0.50		0.50	0.50	0.50	0.50		
Increm. delay d2	0.0		344.9		0.8	1.2	0.9	0.3		
PF factor	1.000		1.000		1.000	1.000	1.000	1.000		
Control delay	27.8		384.9		29.6	28.1	37.4	11.6		
Lane group LOS	C		F		C	C	D	B		
Apprch. delay			384.9		29.3		13.2			
Approach LOS			F		C		B			
Intersec. delay	37.7		Intersection LOS				D			

CAPACITY AND LOS WORKSHEET											
General Information											
Project Description <i>Perris & Kalmia (1Y 2011 PM) - [6 lanes improvement]</i>											
Capacity Analysis											
	EB		WB		NB			SB			
Lane group		TR		LTR		LT	R	L	TR		
Adj. flow rate		0		91		556	141	58	795		
Satflow rate		1863		203		5085	1583	1770	5085		
Lost time		2.0		2.0		2.0	2.0	2.0	2.0		
Green ratio		0.30		0.30		0.35	0.35	0.22	0.60		
Lane group cap.		567		62		1769	551	385	3051		
v/c ratio		0.00		1.47		0.31	0.26	0.15	0.26		
Flow ratio		0.00		0.45		0.11	0.09	0.03	0.16		
Crit. lane group		N		Y		N	N	N	Y		
Sum flow ratios	0.60										
Lost time/cycle	11.00										
Critical v/c ratio	0.67										
Lane Group Capacity, Control Delay, and LOS Determination											
	EB		WB		NB			SB			
Lane group		TR		LTR		LT	R	L	TR		
Adj. flow rate		0		91		556	141	58	795		
Lane group cap.		567		62		1769	551	385	3051		
v/c ratio		0.00		1.47		0.31	0.26	0.15	0.26		
Green ratio		0.30		0.30		0.35	0.35	0.22	0.60		
Unif. delay d1		27.8		40.0		27.5	26.8	36.4	10.9		
Delay factor k		0.50		0.50		0.50	0.50	0.50	0.50		
Increm. delay d2		0.0		279.2		0.5	1.1	0.8	0.2		
PF factor		1.000		1.000		1.000	1.000	1.000	1.000		
Control delay		27.8		319.2		27.9	28.0	37.2	11.1		
Lane group LOS		C		F		C	C	D	B		
Apprch. delay			319.2		27.9			12.9			
Approach LOS			F		C			B			
Intersec. delay	36.3		Intersection LOS					D			

CAPACITY AND LOS WORKSHEET										
General Information										
Project Description <i>Perris & Kalmia (BO 2050 PM) - [4 lanes improvement]</i>										
Capacity Analysis										
	EB		WB		NB		SB			
Lane group	TR		LTR		LT	R	L	TR		
Adj. flow rate	0		99		976	154	66	1124		
Satflow rate	1863		176		3539	1583	1770	3539		
Lost time	2.0		2.0		2.0	2.0	2.0	2.0		
Green ratio	0.30		0.30		0.35	0.35	0.22	0.60		
Lane group cap.	567		54		1231	551	385	2123		
v/c ratio	0.00		1.83		0.79	0.28	0.17	0.53		
Flow ratio	0.00		0.56		0.28	0.10	0.04	0.32		
Crit. lane group	N		Y		N	N	N	Y		
Sum flow ratios	0.88									
Lost time/cycle	11.00									
Critical v/c ratio	0.97									
Lane Group Capacity, Control Delay, and LOS Determination										
	EB		WB		NB		SB			
Lane group	TR		LTR		LT	R	L	TR		
Adj. flow rate	0		99		976	154	66	1124		
Lane group cap.	567		54		1231	551	385	2123		
v/c ratio	0.00		1.83		0.79	0.28	0.17	0.53		
Green ratio	0.30		0.30		0.35	0.35	0.22	0.60		
Unif. delay d1	27.8		40.0		33.8	27.1	36.6	13.5		
Delay factor k	0.50		0.50		0.50	0.50	0.50	0.50		
Increm. delay d2	0.0		437.8		5.3	1.3	1.0	0.9		
PF factor	1.000		1.000		1.000	1.000	1.000	1.000		
Control delay	27.8		477.8		39.1	28.4	37.5	14.4		
Lane group LOS	C		F		D	C	D	B		
Apprch. delay			477.8		37.6		15.7			
Approach LOS			F		D		B			
Intersec. delay	44.9		Intersection LOS				D			

CAPACITY AND LOS WORKSHEET										
General Information										
Project Description <i>Perris & Kalmia (IY 2030 PM) - [4 lanes improvement]</i>										
Capacity Analysis										
	EB		WB		NB		SB			
Lane group		TR		LTR		LT	R	L	TR	
Adj. flow rate		0		94		767	147	62	960	
Satflow rate		1863		191		3539	1583	1770	3539	
Lost time		2.0		2.0		2.0	2.0	2.0	2.0	
Green ratio		0.30		0.30		0.35	0.35	0.22	0.60	
Lane group cap.		567		58		1231	551	385	2123	
v/c ratio		0.00		1.62		0.62	0.27	0.16	0.45	
Flow ratio		0.00		0.49		0.22	0.09	0.04	0.27	
Crit. lane group		N		Y		N	N	N	Y	
Sum flow ratios	0.76									
Lost time/cycle	11.00									
Critical v/c ratio	0.84									
Lane Group Capacity, Control Delay, and LOS Determination										
	EB		WB		NB		SB			
Lane group		TR		LTR		LT	R	L	TR	
Adj. flow rate		0		94		767	147	62	960	
Lane group cap.		567		58		1231	551	385	2123	
v/c ratio		0.00		1.62		0.62	0.27	0.16	0.45	
Green ratio		0.30		0.30		0.35	0.35	0.22	0.60	
Unif. delay d1		27.8		40.0		31.2	27.0	36.5	12.6	
Delay factor k		0.50		0.50		0.50	0.50	0.50	0.50	
Increm. delay d2		0.0		344.9		2.4	1.2	0.9	0.7	
PF factor		1.000		1.000		1.000	1.000	1.000	1.000	
Control delay		27.8		384.9		33.6	28.1	37.4	13.3	
Lane group LOS		C		F		C	C	D	B	
Approch. delay			384.9		32.7		14.8			
Approach LOS			F		C		B			
Intersec. delay	40.0		Intersection LOS				D			

CAPACITY AND LOS WORKSHEET											
General Information											
Project Description <i>Perris & Kalmia (IY 2011 PM) - [4 lanes improvement]</i>											
Capacity Analysis											
	EB		WB		NB			SB			
Lane group		TR		LTR		LT	R	L	TR		
Adj. flow rate		0		91		556	141	58	795		
Satflow rate		1863		203		3539	1583	1770	3539		
Lost time		2.0		2.0		2.0	2.0	2.0	2.0		
Green ratio		0.30		0.30		0.35	0.35	0.22	0.60		
Lane group cap.		567		62		1231	551	385	2123		
v/c ratio		0.00		1.47		0.45	0.26	0.15	0.37		
Flow ratio		0.00		0.45		0.16	0.09	0.03	0.22		
Crit. lane group		N		Y		N	N	N	Y		
Sum flow ratios	0.67										
Lost time/cycle	11.00										
Critical v/c ratio	0.74										
Lane Group Capacity, Control Delay, and LOS Determination											
	EB		WB		NB			SB			
Lane group		TR		LTR		LT	R	L	TR		
Adj. flow rate		0		91		556	141	58	795		
Lane group cap.		567		62		1231	551	385	2123		
v/c ratio		0.00		1.47		0.45	0.26	0.15	0.37		
Green ratio		0.30		0.30		0.35	0.35	0.22	0.60		
Unif. delay d1		27.8		40.0		29.0	26.8	36.4	11.9		
Delay factor k		0.50		0.50		0.50	0.50	0.50	0.50		
Increm. delay d2		0.0		279.2		1.2	1.1	0.8	0.5		
PF factor		1.000		1.000		1.000	1.000	1.000	1.000		
Control delay		27.8		319.2		30.2	28.0	37.2	12.4		
Lane group LOS		C		F		C	C	D	B		
Apprch. delay			319.2		29.8			14.1			
Approach LOS			F		C			B			
Intersec. delay	37.6		Intersection LOS					D			

CAPACITY AND LOS WORKSHEET										
General Information										
Project Description <i>Perris Blvd Street Widening - Perris & Kalmia (BO 2050 PM)</i>										
Capacity Analysis										
	EB		WB		NB		SB			
Lane group		TR		LTR		LT	R	L	TR	
Adj. flow rate		0		99		976	154	66	1124	
Satflow rate		1863		176		1863	1583	1770	1863	
Lost time		2.0		2.0		2.0	2.0	2.0	2.0	
Green ratio		0.30		0.30		0.35	0.35	0.22	0.60	
Lane group cap.		567		54		648	551	385	1118	
v/c ratio		0.00		1.83		1.51	0.28	0.17	1.01	
Flow ratio		0.00		0.56		0.52	0.10	0.04	0.60	
Crit. lane group		N		Y		N	N	N	Y	
Sum flow ratios	1.17									
Lost time/cycle	11.00									
Critical v/c ratio	1.29									
Lane Group Capacity, Control Delay, and LOS Determination										
	EB		WB		NB		SB			
Lane group		TR		LTR		LT	R	L	TR	
Adj. flow rate		0		99		976	154	66	1124	
Lane group cap.		567		54		648	551	385	1118	
v/c ratio		0.00		1.83		1.51	0.28	0.17	1.01	
Green ratio		0.30		0.30		0.35	0.35	0.22	0.60	
Unif. delay d1		27.8		40.0		37.5	27.1	36.6	23.0	
Delay factor k		0.50		0.50		0.50	0.50	0.50	0.50	
Increm. delay d2		0.0		437.8		235.8	1.3	1.0	28.2	
PF factor		1.000		1.000		1.000	1.000	1.000	1.000	
Control delay		27.8		477.8		273.3	28.4	37.5	51.2	
Lane group LOS		C		F		F	C	D	D	
Apprch. delay			477.8		239.9		50.5			
Approach LOS			F		F		D			
Intersec. delay	156.4		Intersection LOS				F			

CAPACITY AND LOS WORKSHEET											
General Information											
Project Description <i>Perris Blvd Street Widening - Perris & Kalmia (1Y 2030 PM)</i>											
Capacity Analysis											
	EB		WB		NB		SB				
Lane group		TR		LTR		LT	R	L	TR		
Adj. flow rate		0		94		767	147	62	960		
Satflow rate		1863		191		1863	1583	1770	1863		
Lost time		2.0		2.0		2.0	2.0	2.0	2.0		
Green ratio		0.30		0.30		0.35	0.35	0.22	0.60		
Lane group cap.		567		58		648	551	385	1118		
v/c ratio		0.00		1.62		1.18	0.27	0.16	0.86		
Flow ratio		0.00		0.49		0.41	0.09	0.04	0.52		
Crit. lane group		N		Y		N	N	N	Y		
Sum flow ratios	1.01										
Lost time/cycle	11.00										
Critical v/c ratio	1.11										
Lane Group Capacity, Control Delay, and LOS Determination											
	EB		WB		NB		SB				
Lane group		TR		LTR		LT	R	L	TR		
Adj. flow rate		0		94		767	147	62	960		
Lane group cap.		567		58		648	551	385	1118		
v/c ratio		0.00		1.62		1.18	0.27	0.16	0.86		
Green ratio		0.30		0.30		0.35	0.35	0.22	0.60		
Unif. delay d1		27.8		40.0		37.5	27.0	36.5	19.0		
Delay factor k		0.50		0.50		0.50	0.50	0.50	0.50		
Increm. delay d2		0.0		344.9		97.8	1.2	0.9	8.6		
PF factor		1.000		1.000		1.000	1.000	1.000	1.000		
Control delay		27.8		384.9		135.3	28.1	37.4	27.6		
Lane group LOS		C		F		F	C	D	C		
Apprch. delay			384.9		118.0		28.2				
Approach LOS			F		F		C				
Intersec. delay	85.2		Intersection LOS					F			

CAPACITY AND LOS WORKSHEET										
General Information										
Project Description <i>Perris Blvd Street Widening - Perris & Kalmia (IY 2011 PM)</i>										
Capacity Analysis										
	EB		WB		NB		SB			
Lane group		TR		LTR		LT	R	L	TR	
Adj. flow rate		0		91		556	141	58	795	
Satflow rate		1863		203		1863	1583	1770	1863	
Lost time		2.0		2.0		2.0	2.0	2.0	2.0	
Green ratio		0.30		0.30		0.35	0.35	0.22	0.60	
Lane group cap.		567		62		648	551	385	1118	
v/c ratio		0.00		1.47		0.86	0.26	0.15	0.71	
Flow ratio		0.00		0.45		0.30	0.09	0.03	0.43	
Crit. lane group		N		Y		N	N	N	Y	
Sum flow ratios	0.88									
Lost time/cycle	11.00									
Critical v/c ratio	0.97									
Lane Group Capacity, Control Delay, and LOS Determination										
	EB		WB		NB		SB			
Lane group		TR		LTR		LT	R	L	TR	
Adj. flow rate		0		91		556	141	58	795	
Lane group cap.		567		62		648	551	385	1118	
v/c ratio		0.00		1.47		0.86	0.26	0.15	0.71	
Green ratio		0.30		0.30		0.35	0.35	0.22	0.60	
Unif. delay d1		27.8		40.0		34.9	26.8	36.4	16.0	
Delay factor k		0.50		0.50		0.50	0.50	0.50	0.50	
Increm. delay d2		0.0		279.2		13.8	1.1	0.8	3.8	
PF factor		1.000		1.000		1.000	1.000	1.000	1.000	
Control delay		27.8		319.2		48.7	28.0	37.2	19.9	
Lane group LOS		C		F		D	C	D	B	
Apprch. delay			319.2		44.5		21.1			
Approach LOS			F		D		C			
Intersec. delay	47.5		Intersection LOS				D			

CAPACITY AND LOS WORKSHEET										
General Information										
Project Description <i>Perris Blvd Street Widening - Perris & Kalmia (EX PM)</i>										
Capacity Analysis										
	EB		WB		NB		SB			
Lane group		TR		LTR		LT	R	L	TR	
Adj. flow rate		0		90		512	140	57	760	
Satflow rate		1863		209		1863	1583	1770	1863	
Lost time		2.0		2.0		2.0	2.0	2.0	2.0	
Green ratio		0.30		0.30		0.35	0.35	0.22	0.60	
Lane group cap.		567		64		648	551	385	1118	
v/c ratio		0.00		1.41		0.79	0.25	0.15	0.68	
Flow ratio		0.00		0.43		0.27	0.09	0.03	0.41	
Crit. lane group		N		Y		N	N	N	Y	
Sum flow ratios	0.84									
Lost time/cycle	11.00									
Critical v/c ratio	0.93									
Lane Group Capacity, Control Delay, and LOS Determination										
	EB		WB		NB		SB			
Lane group		TR		LTR		LT	R	L	TR	
Adj. flow rate		0		90		512	140	57	760	
Lane group cap.		567		64		648	551	385	1118	
v/c ratio		0.00		1.41		0.79	0.25	0.15	0.68	
Green ratio		0.30		0.30		0.35	0.35	0.22	0.60	
Unif. delay d1		27.8		40.0		33.7	26.8	36.4	15.5	
Delay factor k		0.50		0.50		0.50	0.50	0.50	0.50	
Increm. delay d2		0.0		253.1		9.5	1.1	0.8	3.3	
PF factor		1.000		1.000		1.000	1.000	1.000	1.000	
Control delay		27.8		293.1		43.2	27.9	37.2	18.9	
Lane group LOS		C		F		D	C	D	B	
Apprch. delay			293.1		39.9		20.2			
Approach LOS			F		D		C			
Intersec. delay	44.2		Intersection LOS				D			

CAPACITY AND LOS WORKSHEET												
General Information												
Project Description <i>Perris & Kalmia (BO 2050 PM) - [6 lanes improvement, pocket]</i>												
Capacity Analysis												
	EB			WB			NB			SB		
Lane group				L	TR			LT	R	L	TR	
Adj. flow rate				71	28			976	154	66	1124	
Satflow rate				1770	1583			5085	1583	1770	5085	
Lost time				2.0	2.0			2.0	2.0	2.0	2.0	
Green ratio				0.30	0.30			0.35	0.35	0.22	0.60	
Lane group cap.				539	482			1769	551	385	3051	
v/c ratio				0.13	0.06			0.55	0.28	0.17	0.37	
Flow ratio				0.04	0.02			0.19	0.10	0.04	0.22	
Crit. lane group				Y	N			Y	N	Y	N	
Sum flow ratios	0.27											
Lost time/cycle	15.00											
Critical v/c ratio	0.31											
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
Lane group				L	TR			LT	R	L	TR	
Adj. flow rate				71	28			976	154	66	1124	
Lane group cap.				539	482			1769	551	385	3051	
v/c ratio				0.13	0.06			0.55	0.28	0.17	0.37	
Green ratio				0.30	0.30			0.35	0.35	0.22	0.60	
Unif. delay d1				29.0	28.3			30.3	27.1	36.6	11.8	
Delay factor k				0.50	0.50			0.50	0.50	0.50	0.50	
Increm. delay d2				0.5	0.2			1.2	1.3	1.0	0.3	
PF factor				1.000	1.000			1.000	1.000	1.000	1.000	
Control delay				29.5	28.6			31.5	28.4	37.5	12.2	
Lane group LOS				C	C			C	C	D	B	
Approch. delay				29.2			31.1			13.6		
Approach LOS				C			C			B		
Intersec. delay	22.4			Intersection LOS						C		

CAPACITY AND LOS WORKSHEET												
General Information												
Project Description <i>Perris & Kalmia (1Y 2030 PM) - [6 lanes improvement, pocket]</i>												
Capacity Analysis												
	EB			WB			NB			SB		
Lane group				L	TR			LT	R	L	TR	
Adj. flow rate				67	27			767	147	62	960	
Satflow rate				1770	1583			5085	1583	1770	5085	
Lost time				2.0	2.0			2.0	2.0	2.0	2.0	
Green ratio				0.30	0.30			0.35	0.35	0.22	0.60	
Lane group cap.				539	482			1769	551	385	3051	
v/c ratio				0.12	0.06			0.43	0.27	0.16	0.31	
Flow ratio				0.04	0.02			0.15	0.09	0.04	0.19	
Crit. lane group				Y	N			N	N	N	Y	
Sum flow ratios				0.23								
Lost time/cycle				11.00								
Critical v/c ratio				0.25								
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
Lane group				L	TR			LT	R	L	TR	
Adj. flow rate				67	27			767	147	62	960	
Lane group cap.				539	482			1769	551	385	3051	
v/c ratio				0.12	0.06			0.43	0.27	0.16	0.31	
Green ratio				0.30	0.30			0.35	0.35	0.22	0.60	
Unif. delay d1				28.9	28.3			28.8	27.0	36.5	11.3	
Delay factor k				0.50	0.50			0.50	0.50	0.50	0.50	
Increm. delay d2				0.5	0.2			0.8	1.2	0.9	0.3	
PF factor				1.000	1.000			1.000	1.000	1.000	1.000	
Control delay				29.4	28.5			29.6	28.1	37.4	11.6	
Lane group LOS				C	C			C	C	D	B	
Approch. delay				29.1			29.3			13.2		
Approach LOS				C			C			B		
Intersec. delay	21.2			Intersection LOS						C		

CAPACITY AND LOS WORKSHEET												
General Information												
Project Description <i>Perris & Kalmia (1Y 2011 PM) - [6 lanes improvement, pocket]</i>												
Capacity Analysis												
	EB			WB			NB			SB		
Lane group				L	TR			LT	R	L	TR	
Adj. flow rate				65	26			556	141	58	795	
Satflow rate				1770	1583			5085	1583	1770	5085	
Lost time				2.0	2.0			2.0	2.0	2.0	2.0	
Green ratio				0.30	0.30			0.35	0.35	0.22	0.60	
Lane group cap.				539	482			1769	551	385	3051	
v/c ratio				0.12	0.05			0.31	0.26	0.15	0.26	
Flow ratio				0.04	0.02			0.11	0.09	0.03	0.16	
Crit. lane group				Y	N			N	N	N	Y	
Sum flow ratios	0.19											
Lost time/cycle	11.00											
Critical v/c ratio	0.21											
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
Lane group				L	TR			LT	R	L	TR	
Adj. flow rate				65	26			556	141	58	795	
Lane group cap.				539	482			1769	551	385	3051	
v/c ratio				0.12	0.05			0.31	0.26	0.15	0.26	
Green ratio				0.30	0.30			0.35	0.35	0.22	0.60	
Unif. delay d1				28.9	28.3			27.5	26.8	36.4	10.9	
Delay factor k				0.50	0.50			0.50	0.50	0.50	0.50	
Increm. delay d2				0.5	0.2			0.5	1.1	0.8	0.2	
PF factor				1.000	1.000			1.000	1.000	1.000	1.000	
Control delay				29.3	28.5			27.9	28.0	37.2	11.1	
Lane group LOS				C	C			C	C	D	B	
Apprch. delay				29.1			27.9			12.9		
Approach LOS				C			C			B		
Intersec. delay	20.2			Intersection LOS						C		

CAPACITY AND LOS WORKSHEET												
General Information												
Project Description <i>Perris & Kalmia (BO 2050 PM) - [4 lanes improvement, pocket]</i>												
Capacity Analysis												
	EB			WB			NB			SB		
Lane group				L	TR			LT	R	L	TR	
Adj. flow rate				71	28			976	154	66	1124	
Satflow rate				1770	1583			3539	1583	1770	3539	
Lost time				2.0	2.0			2.0	2.0	2.0	2.0	
Green ratio				0.30	0.30			0.35	0.35	0.22	0.60	
Lane group cap.				539	482			1231	551	385	2123	
v/c ratio				0.13	0.06			0.79	0.28	0.17	0.53	
Flow ratio				0.04	0.02			0.28	0.10	0.04	0.32	
Crit. lane group				Y	N			N	N	N	Y	
Sum flow ratios				0.36								
Lost time/cycle				11.00								
Critical v/c ratio				0.40								
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
Lane group				L	TR			LT	R	L	TR	
Adj. flow rate				71	28			976	154	66	1124	
Lane group cap.				539	482			1231	551	385	2123	
v/c ratio				0.13	0.06			0.79	0.28	0.17	0.53	
Green ratio				0.30	0.30			0.35	0.35	0.22	0.60	
Unif. delay d1				29.0	28.3			33.8	27.1	36.6	13.5	
Delay factor k				0.50	0.50			0.50	0.50	0.50	0.50	
Increm. delay d2				0.5	0.2			5.3	1.3	1.0	0.9	
PF factor				1.000	1.000			1.000	1.000	1.000	1.000	
Control delay				29.5	28.6			39.1	28.4	37.5	14.4	
Lane group LOS				C	C			D	C	D	B	
Apprch. delay				29.2			37.6			15.7		
Approach LOS				C			D			B		
Intersec. delay	26.5			Intersection LOS						C		

CAPACITY AND LOS WORKSHEET												
General Information												
Project Description <i>Perris & Kalmia (IY 2030 PM) - [4 lanes improvement, pocket]</i>												
Capacity Analysis												
	EB			WB			NB			SB		
Lane group				L	TR			LT	R	L	TR	
Adj. flow rate				67	27			767	147	62	960	
Satflow rate				1770	1583			3539	1583	1770	3539	
Lost time				2.0	2.0			2.0	2.0	2.0	2.0	
Green ratio				0.30	0.30			0.35	0.35	0.22	0.60	
Lane group cap.				539	482			1231	551	385	2123	
v/c ratio				0.12	0.06			0.62	0.27	0.16	0.45	
Flow ratio				0.04	0.02			0.22	0.09	0.04	0.27	
Crit. lane group				Y	N			N	N	N	Y	
Sum flow ratios	0.31											
Lost time/cycle	11.00											
Critical v/c ratio	0.34											
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
Lane group				L	TR			LT	R	L	TR	
Adj. flow rate				67	27			767	147	62	960	
Lane group cap.				539	482			1231	551	385	2123	
v/c ratio				0.12	0.06			0.62	0.27	0.16	0.45	
Green ratio				0.30	0.30			0.35	0.35	0.22	0.60	
Unif. delay d1				28.9	28.3			31.2	27.0	36.5	12.6	
Delay factor k				0.50	0.50			0.50	0.50	0.50	0.50	
Increm. delay d2				0.5	0.2			2.4	1.2	0.9	0.7	
PF factor				1.000	1.000			1.000	1.000	1.000	1.000	
Control delay				29.4	28.5			33.6	28.1	37.4	13.3	
Lane group LOS				C	C			C	C	D	B	
Apprch. delay				29.1			32.7			14.8		
Approach LOS				C			C			B		
Intersec. delay	23.5			Intersection LOS						C		

CAPACITY AND LOS WORKSHEET												
General Information												
Project Description <i>Perris & Kalmia (1Y 2011 PM) - [4 lanes improvement, pocket]</i>												
Capacity Analysis												
	EB			WB			NB			SB		
Lane group				L	TR			LT	R	L	TR	
Adj. flow rate				65	26			556	141	58	795	
Satflow rate				1770	1583			3539	1583	1770	3539	
Lost time				2.0	2.0			2.0	2.0	2.0	2.0	
Green ratio				0.30	0.30			0.35	0.35	0.22	0.60	
Lane group cap.				539	482			1231	551	385	2123	
v/c ratio				0.12	0.05			0.45	0.26	0.15	0.37	
Flow ratio				0.04	0.02			0.16	0.09	0.03	0.22	
Crit. lane group				Y	N			N	N	N	Y	
Sum flow ratios	0.26											
Lost time/cycle	11.00											
Critical v/c ratio	0.29											
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
Lane group				L	TR			LT	R	L	TR	
Adj. flow rate				65	26			556	141	58	795	
Lane group cap.				539	482			1231	551	385	2123	
v/c ratio				0.12	0.05			0.45	0.26	0.15	0.37	
Green ratio				0.30	0.30			0.35	0.35	0.22	0.60	
Unif. delay d1				28.9	28.3			29.0	26.8	36.4	11.9	
Delay factor k				0.50	0.50			0.50	0.50	0.50	0.50	
Increm. delay d2				0.5	0.2			1.2	1.1	0.8	0.5	
PF factor				1.000	1.000			1.000	1.000	1.000	1.000	
Control delay				29.3	28.5			30.2	28.0	37.2	12.4	
Lane group LOS				C	C			C	C	D	B	
Apprch. delay				29.1			29.8			14.1		
Approach LOS				C			C			B		
Intersec. delay	21.6			Intersection LOS						C		

CAPACITY AND LOS WORKSHEET												
General Information												
Project Description <i>Perris & Ironwood (BO 2050 PM) - [6 lanes total]</i>												
Capacity Analysis												
	EB			WB			NB			SB		
Lane group	L	T	R	L	T	R	L	TR		L	TR	
Adj. flow rate	105	385	363	162	256	151	347	1093		157	1123	
Satflow rate	1770	1863	1583	1770	1863	1583	1770	4946		1770	5028	
Lost time	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0		2.0	2.0	
Green ratio	0.17	0.25	0.25	0.17	0.25	0.25	0.17	0.25		0.17	0.25	
Lane group cap.	296	468	397	296	468	397	296	1242		296	1262	
v/c ratio	0.35	0.82	0.91	0.55	0.55	0.38	1.17	0.88		0.53	0.89	
Flow ratio	0.06	0.21	0.23	0.09	0.14	0.10	0.20	0.22		0.09	0.22	
Crit. lane group	N	N	Y	Y	N	N	Y	N		N	Y	
Sum flow ratios	0.74											
Lost time/cycle	19.50											
Critical v/c ratio	0.88											
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
Lane group	L	T	R	L	T	R	L	TR		L	TR	
Adj. flow rate	105	385	363	162	256	151	347	1093		157	1123	
Lane group cap.	296	468	397	296	468	397	296	1242		296	1262	
v/c ratio	0.35	0.82	0.91	0.55	0.55	0.38	1.17	0.88		0.53	0.89	
Green ratio	0.17	0.25	0.25	0.17	0.25	0.25	0.17	0.25		0.17	0.25	
Unif. delay d1	44.0	42.2	43.5	45.6	38.9	37.1	49.8	43.0		45.5	43.2	
Delay factor k	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50		0.50	0.50	
Increm. delay d2	3.3	15.0	28.0	7.1	4.5	2.8	107.4	9.1		6.7	9.6	
PF factor	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000		1.000	1.000	
Control delay	47.3	57.3	71.5	52.7	43.4	39.8	157.2	52.1		52.1	52.8	
Lane group LOS	D	E	E	D	D	D	F	D		D	D	
Apprch. delay	62.1			45.1			77.4			52.7		
Approach LOS	E			D			E			D		
Intersec. delay	62.2			Intersection LOS						E		

CAPACITY AND LOS WORKSHEET												
General Information												
Project Description <i>Perris & Ironwood (IY 2030 PM) - [6 lanes total]</i>												
Capacity Analysis												
Lane group	EB			WB			NB			SB		
	L	T	R	L	T	R	L	TR		L	TR	
Adj. flow rate	91	311	312	156	223	133	313	911		151	965	
Satflow rate	1770	1863	1583	1770	1863	1583	1770	4925		1770	5022	
Lost time	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0		2.0	2.0	
Green ratio	0.17	0.25	0.25	0.17	0.25	0.25	0.17	0.25		0.17	0.25	
Lane group cap.	296	468	397	296	468	397	296	1236		296	1261	
v/c ratio	0.31	0.66	0.79	0.53	0.48	0.34	1.06	0.74		0.51	0.77	
Flow ratio	0.05	0.17	0.20	0.09	0.12	0.08	0.18	0.18		0.09	0.19	
Crit. lane group	N	N	Y	Y	N	N	Y	N		N	Y	
Sum flow ratios	0.65											
Lost time/cycle	19.50											
Critical v/c ratio	0.78											
Lane Group Capacity, Control Delay, and LOS Determination												
Lane group	EB			WB			NB			SB		
	L	T	R	L	T	R	L	TR		L	TR	
Adj. flow rate	91	311	312	156	223	133	313	911		151	965	
Lane group cap.	296	468	397	296	468	397	296	1236		296	1261	
v/c ratio	0.31	0.66	0.79	0.53	0.48	0.34	1.06	0.74		0.51	0.77	
Green ratio	0.17	0.25	0.25	0.17	0.25	0.25	0.17	0.25		0.17	0.25	
Unif. delay d1	43.7	40.2	41.8	45.4	38.1	36.6	49.8	41.1		45.3	41.5	
Delay factor k	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50		0.50	0.50	
Increm. delay d2	2.7	7.3	14.5	6.6	3.5	2.3	68.2	4.0		6.2	4.5	
PF factor	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000		1.000	1.000	
Control delay	46.3	47.5	56.2	52.0	41.5	38.9	118.0	45.1		51.5	46.0	
Lane group LOS	D	D	E	D	D	D	F	D		D	D	
Approch. delay	51.2			44.0			63.7			46.7		
Approach LOS	D			D			E			D		
Intersec. delay	53.1			Intersection LOS						D		

CAPACITY AND LOS WORKSHEET												
General Information												
Project Description <i>Perris & Ironwood (IY 2011 PM) - [6 lanes total]</i>												
Capacity Analysis												
	EB			WB			NB			SB		
Lane group	L	T	R	L	T	R	L	TR		L	TR	
Adj. flow rate	76	235	259	148	191	115	277	728		144	806	
Satflow rate	1770	1863	1583	1770	1863	1583	1770	4894		1770	5012	
Lost time	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0		2.0	2.0	
Green ratio	0.17	0.25	0.25	0.17	0.25	0.25	0.17	0.25		0.17	0.25	
Lane group cap.	296	468	397	296	468	397	296	1229		296	1258	
v/c ratio	0.26	0.50	0.65	0.50	0.41	0.29	0.94	0.59		0.49	0.64	
Flow ratio	0.04	0.13	0.16	0.08	0.10	0.07	0.16	0.15		0.08	0.16	
Crit. lane group	N	N	Y	Y	N	N	Y	N		N	Y	
Sum flow ratios	0.56											
Lost time/cycle	19.50											
Critical v/c ratio	0.67											
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
Lane group	L	T	R	L	T	R	L	TR		L	TR	
Adj. flow rate	76	235	259	148	191	115	277	728		144	806	
Lane group cap.	296	468	397	296	468	397	296	1229		296	1258	
v/c ratio	0.26	0.50	0.65	0.50	0.41	0.29	0.94	0.59		0.49	0.64	
Green ratio	0.17	0.25	0.25	0.17	0.25	0.25	0.17	0.25		0.17	0.25	
Unif. delay d1	43.3	38.4	40.1	45.2	37.3	36.1	49.1	39.4		45.1	39.9	
Delay factor k	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50		0.50	0.50	
Increm. delay d2	2.1	3.8	8.1	5.9	2.6	1.8	38.2	2.1		5.6	2.5	
PF factor	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000		1.000	1.000	
Control delay	45.4	42.2	48.2	51.1	40.0	38.0	87.3	41.5		50.7	42.5	
Lane group LOS	D	D	D	D	D	D	F	D		D	D	
Apprch. delay	45.3			43.1			54.1			43.7		
Approach LOS	D			D			D			D		
Intersec. delay	47.4			Intersection LOS						D		

Perris Boulevard/Pico Vista Way
Intersection LOS Analysis

CAPACITY AND LOS WORKSHEET											
General Information											
Project Description <i>Perris Blvd & Pico Vista Wy (BO 2050 PM) (6 lanes total)</i>											
Capacity Analysis											
	EB			WB			NB			SB	
Lane group				LTR			TR			L	T
Adj. flow rate				6			1107			65	1101
Satflow rate				1729			5081			1805	5187
Lost time				2.0			2.0			2.0	2.0
Green ratio				0.30			0.35			0.22	0.60
Lane group cap.				526			1767			392	3112
v/c ratio				0.01			0.63			0.17	0.35
Flow ratio				0.00			0.22			0.04	0.21
Crit. lane group				Y			Y			Y	N
Sum flow ratios	0.26										
Lost time/cycle	15.00										
Critical v/c ratio	0.30										
Lane Group Capacity, Control Delay, and LOS Determination											
	EB			WB			NB			SB	
Lane group				LTR			TR			L	T
Adj. flow rate				6			1107			65	1101
Lane group cap.				526			1767			392	3112
v/c ratio				0.01			0.63			0.17	0.35
Green ratio				0.30			0.35			0.22	0.60
Unif. delay d1				27.9			31.3			36.5	11.7
Delay factor k				0.50			0.50			0.50	0.50
Increm. delay d2				0.0			1.7			0.9	0.3
PF factor				1.000			1.000			1.000	1.000
Control delay				28.0			33.0			37.4	12.0
Lane group LOS				C			C			D	B
Apprch. delay				28.0			33.0			13.4	
Approach LOS				C			C			B	
Intersec. delay	22.9			Intersection LOS						C	

CAPACITY AND LOS WORKSHEET											
General Information											
Project Description <i>Perris Blvd & Pico Vista Wy (1Y 2030 PM) (6 lanes total)</i>											
Capacity Analysis											
	EB			WB			NB			SB	
Lane group				LTR			TR			L	T
Adj. flow rate				6			739			4	987
Satflow rate				1729			5175			1805	5187
Lost time				2.0			2.0			2.0	2.0
Green ratio				0.30			0.35			0.22	0.60
Lane group cap.				526			1800			392	3112
v/c ratio				0.01			0.41			0.01	0.32
Flow ratio				0.00			0.14			0.00	0.19
Crit. lane group				Y			N			N	Y
Sum flow ratios	0.19										
Lost time/cycle	11.00										
Critical v/c ratio	0.21										
Lane Group Capacity, Control Delay, and LOS Determination											
	EB			WB			NB			SB	
Lane group				LTR			TR			L	T
Adj. flow rate				6			739			4	987
Lane group cap.				526			1800			392	3112
v/c ratio				0.01			0.41			0.01	0.32
Green ratio				0.30			0.35			0.22	0.60
Unif. delay d1				27.9			28.5			35.3	11.4
Delay factor k				0.50			0.50			0.50	0.50
Increm. delay d2				0.0			0.7			0.0	0.3
PF factor				1.000			1.000			1.000	1.000
Control delay				28.0			29.2			35.3	11.6
Lane group LOS				C			C			D	B
Apprch. delay				28.0			29.2			11.7	
Approach LOS				C			C			B	
Intersec. delay	19.2			Intersection LOS						B	

CAPACITY AND LOS WORKSHEET

General Information

Project Description *Perris Blvd & Pico Vista Wy (IY 2011 PM) (6 lanes total)*

Capacity Analysis

	EB			WB			NB			SB		
Lane group				LTR			TR			L	T	
Adj. flow rate				6			536			4	820	
Satflow rate				1729			5178			1805	5187	
Lost time				2.0			2.0			2.0	2.0	
Green ratio				0.30			0.35			0.22	0.60	
Lane group cap.				526			1801			392	3112	
v/c ratio				0.01			0.30			0.01	0.26	
Flow ratio				0.00			0.10			0.00	0.16	
Crit. lane group				Y			N			N	Y	
Sum flow ratios	0.16											
Lost time/cycle	11.00											
Critical v/c ratio	0.18											

Lane Group Capacity, Control Delay, and LOS Determination

	EB			WB			NB			SB		
Lane group				LTR			TR			L	T	
Adj. flow rate				6			536			4	820	
Lane group cap.				526			1801			392	3112	
v/c ratio				0.01			0.30			0.01	0.26	
Green ratio				0.30			0.35			0.22	0.60	
Unif. delay d1				27.9			27.3			35.3	10.9	
Delay factor k				0.50			0.50			0.50	0.50	
Increm. delay d2				0.0			0.4			0.0	0.2	
PF factor				1.000			1.000			1.000	1.000	
Control delay				28.0			27.7			35.3	11.1	
Lane group LOS				C			C			D	B	
Apprch. delay				28.0			27.7			11.3		
Approach LOS				C			C			B		
Intersec. delay	17.8			Intersection LOS						B		

CAPACITY AND LOS WORKSHEET											
General Information											
Project Description <i>Perris Blvd & Pico Vista Wy (IY 2050 PM) (4 lanes total)</i>											
Capacity Analysis											
	EB			WB			NB			SB	
Lane group				LTR			TR		L	T	
Adj. flow rate				6			1107		65	1101	
Satflow rate				1729			3536		1805	3610	
Lost time				2.0			2.0		2.0	2.0	
Green ratio				0.30			0.35		0.22	0.60	
Lane group cap.				526			1230		392	2166	
v/c ratio				0.01			0.90		0.17	0.51	
Flow ratio				0.00			0.31		0.04	0.30	
Crit. lane group				Y			Y		Y	N	
Sum flow ratios							0.35				
Lost time/cycle							15.00				
Critical v/c ratio							0.41				
Lane Group Capacity, Control Delay, and LOS Determination											
	EB			WB			NB			SB	
Lane group				LTR			TR		L	T	
Adj. flow rate				6			1107		65	1101	
Lane group cap.				526			1230		392	2166	
v/c ratio				0.01			0.90		0.17	0.51	
Green ratio				0.30			0.35		0.22	0.60	
Unif. delay d1				27.9			35.6		36.5	13.2	
Delay factor k				0.50			0.50		0.50	0.50	
Increm. delay d2				0.0			10.7		0.9	0.9	
PF factor				1.000			1.000		1.000	1.000	
Control delay				28.0			46.3		37.4	14.1	
Lane group LOS				C			D		D	B	
Apprch. delay				28.0			46.3			15.4	
Approach LOS				C			D			B	
Intersec. delay	30.4			Intersection LOS						C	

CAPACITY AND LOS WORKSHEET											
General Information											
Project Description <i>Perris Blvd & Pico Vista Wy (IY 2030 PM) (4 lanes total)</i>											
Capacity Analysis											
	EB			WB			NB			SB	
Lane group				LTR			TR			L	T
Adj. flow rate				6			739			4	987
Satflow rate				1729			3602			1805	3610
Lost time				2.0			2.0			2.0	2.0
Green ratio				0.30			0.35			0.22	0.60
Lane group cap.				526			1253			392	2166
v/c ratio				0.01			0.59			0.01	0.46
Flow ratio				0.00			0.21			0.00	0.27
Crit. lane group				Y			N			N	Y
Sum flow ratios	0.28										
Lost time/cycle	11.00										
Critical v/c ratio	0.31										
Lane Group Capacity, Control Delay, and LOS Determination											
	EB			WB			NB			SB	
Lane group				LTR			TR			L	T
Adj. flow rate				6			739			4	987
Lane group cap.				526			1253			392	2166
v/c ratio				0.01			0.59			0.01	0.46
Green ratio				0.30			0.35			0.22	0.60
Unif. delay d1				27.9			30.8			35.3	12.7
Delay factor k				0.50			0.50			0.50	0.50
Increm. delay d2				0.0			2.0			0.0	0.7
PF factor				1.000			1.000			1.000	1.000
Control delay				28.0			32.8			35.3	13.4
Lane group LOS				C			C			D	B
Aprpch. delay				28.0			32.8			13.4	
Approach LOS				C			C			B	
Intersec. delay	21.7			Intersection LOS						C	

CAPACITY AND LOS WORKSHEET											
General Information											
Project Description <i>Perris Blvd & Pico Vista Wy (IY 2011 PM) (4 lanes total)</i>											
Capacity Analysis											
	EB			WB			NB			SB	
Lane group				LTR			TR			L	T
Adj. flow rate				6			536			4	820
Satflow rate				1729			3604			1805	3610
Lost time				2.0			2.0			2.0	2.0
Green ratio				0.30			0.35			0.22	0.60
Lane group cap.				526			1254			392	2166
v/c ratio				0.01			0.43			0.01	0.38
Flow ratio				0.00			0.15			0.00	0.23
Crit. lane group				Y			N			N	Y
Sum flow ratios	0.23										
Lost time/cycle	11.00										
Critical v/c ratio	0.26										
Lane Group Capacity, Control Delay, and LOS Determination											
	EB			WB			NB			SB	
Lane group				LTR			TR			L	T
Adj. flow rate				6			536			4	820
Lane group cap.				526			1254			392	2166
v/c ratio				0.01			0.43			0.01	0.38
Green ratio				0.30			0.35			0.22	0.60
Unif. delay d1				27.9			28.7			35.3	11.9
Delay factor k				0.50			0.50			0.50	0.50
Increm. delay d2				0.0			1.1			0.0	0.5
PF factor				1.000			1.000			1.000	1.000
Control delay				28.0			29.8			35.3	12.4
Lane group LOS				C			C			D	B
Apprch. delay				28.0			29.8			12.5	
Approach LOS				C			C			B	
Intersec. delay	19.4			Intersection LOS						B	

TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information	
Analyst	Philip Nitollama	Intersection	Perris (NS) & Pico Vista (EW)
Agency/Co.	Boyle Engineering	Jurisdiction	Moreno Valley
Date Performed	6/20/2007	Analysis Year	Interim Year 2030 PM
Analysis Time Period	Year 2030- No Build	Project ID	Perris Boulevard Widening (JN# 23079.00)
East/West Street: Pico Vista Way		North/South Street: Perris Boulevard	
Intersection Orientation: North-South		Study Period (hrs): 0.25	

Vehicle Volumes and Adjustments

Major Street	Northbound			Southbound			
	Movement	1	2	3	4	5	6
	L	T	R	L	T	R	
Volume	0	706	11	4	957	0	
Peak-Hour Factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97	
Hourly Flow Rate, HFR	0	727	11	4	986	0	
Percent Heavy Vehicles	0	-	-	0	-	-	
Median Type	Undivided						
RT Channelized			0			0	
Lanes	0	1	0	1	1	0	
Configuration			TR	L	T		
Upstream Signal		1			1		
Minor Street	Westbound			Eastbound			
	Movement	7	8	9	10	11	12
	L	T	R	L	T	R	
Volume	3	0	3	0	0	0	
Peak-Hour Factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97	
Hourly Flow Rate, HFR	3	0	3	0	0	0	
Percent Heavy Vehicles	0	0	0	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		Y			N		
Storage		1			0		
RT Channelized			0			0	
Lanes	0	1	0	0	0	0	
Configuration		LTR					

Delay, Queue Length, and Level of Service

Approach	NB	SB	Westbound			Eastbound		
			7	8	9	10	11	12
Movement	1	4						
Lane Configuration		L		LTR				
(vph)		4		6				
C (m) (vph)		891						
y/c		0.00						
5% queue length		0.01						
Control Delay		9.1						
LOS		A						
Approach Delay	--	--						
Approach LOS	--	--						

TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information	
Analyst	Philip Nitollama	Intersection	Perris (NS) & Pico Vista (EW)
Agency/Co.	Boyle Engineering	Jurisdiction	Moreno Valley
Date Performed	6/20/2007	Analysis Year	Buildout 2050 PM
Analysis Time Period	Buildout 2050 PM - No Build	Project ID	Perris Boulevard Widening (JN# 23079.00)
East/West Street: Pico Vista Way		North/South Street: Perris Boulevard	
Intersection Orientation: North-South		Study Period (hrs): 0.25	

Vehicle Volumes and Adjustments

Major Street Movement	Northbound			Southbound		
	1	2	3	4	5	6
	L	T	R	L	T	R
Volume	0	927	146	63	1068	0
Peak-Hour Factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97
Hourly Flow Rate, HFR	0	955	150	64	1101	0
Percent Heavy Vehicles	0	--	--	0	--	--
Median Type	Undivided					
RT Channelized			0			0
Lanes	0	1	0	1	1	0
Configuration			TR	L	T	
Upstream Signal		1			1	
Minor Street Movement	Westbound			Eastbound		
	7	8	9	10	11	12
	L	T	R	L	T	R
Volume	3	0	3	0	0	0
Peak-Hour Factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97
Hourly Flow Rate, HFR	3	0	3	0	0	0
Percent Heavy Vehicles	0	0	0	0	0	0
Percent Grade (%)		0			0	
Flared Approach		Y			N	
Storage		1			0	
RT Channelized			0			0
Lanes	0	1	0	0	0	0
Configuration		LTR				

Delay, Queue Length, and Level of Service

Approach Movement	NB	SB	Westbound			Eastbound		
	1	4	7	8	9	10	11	12
Lane Configuration		L		LTR				
Capacity (vph)		64		6				
Flow (m) (vph)		534						
Control		0.12						
% queue length		0.41						
Control Delay		12.7						
LOS		B						
Approach Delay	--	--						
Approach LOS	--	--						

TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information	
Analyst	Philip Nitollama	Intersection	Perris (NS) & Pico Vista (EW)
Agency/Co.	Boyle Engineering	Jurisdiction	Moreno Valley
Date Performed	6/20/2007	Analysis Year	Interim Year 2011 PM
Analysis Time Period	Year 2011 - No Build	Project ID	Perris Boulevard Widening (JN# 23079.00)

East/West Street: Pico Vista Way	North/South Street: Perris Boulevard
Intersection Orientation: North-South	Study Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street Movement	Northbound			Southbound		
	1 L	2 T	3 R	4 L	5 T	6 R
Volume	0	514	6	4	795	0
Peak-Hour Factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97
Hourly Flow Rate, HFR	0	529	6	4	819	0
Percent Heavy Vehicles	0	--	--	0	--	--
Median Type	Undivided					
RT Channelized			0			0
Lanes	0	1	0	1	1	0
Configuration			TR	L	T	
Upstream Signal		1			1	

Minor Street Movement	Westbound			Eastbound		
	7 L	8 T	9 R	10 L	11 T	12 R
Volume	3	0	3	0	0	0
Peak-Hour Factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97
Hourly Flow Rate, HFR	3	0	3	0	0	0
Percent Heavy Vehicles	0	0	0	0	0	0
Percent Grade (%)	0			0		
Flared Approach	Y			N		
Storage	1			0		
RT Channelized			0			0
Lanes	0	1	0	0	0	0
Configuration	LTR					

Delay, Queue Length, and Level of Service

Approach Movement	NB	SB	Westbound			Eastbound		
	1	4	7	8	9	10	11	12
Lane Configuration		L		LTR				
Volume (vph)		4		6				
Flow (m) (vph)		1052		1180				
Volume/c		0.00		0.01				
5% queue length		0.01		0.02				
Control Delay		8.4		8.1				
LOS		A		A				
Approach Delay	--	--		8.1				
Approach LOS	--	--		A				

TWO-WAY STOP CONTROL SUMMARY								
General Information				Site Information				
Analyst	Philip Nitollama			Intersection	Perris (NS) & Pico Vista (EW)			
Agency/Co.	Boyle Engineering			Jurisdiction	Moreno Valley			
Date Performed	6/20/2007			Analysis Year	Existing PM			
Analysis Time Period	Existing			Project ID	Perris Boulevard Widening (JN# 23079.00)			
East/West Street: Pico Vista Way				North/South Street: Perris Boulevard				
Intersection Orientation: North-South				Study Period (hrs): 0.25				
Vehicle Volumes and Adjustments								
Major Street	Northbound			Southbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume	0	473	5	4	761	0		
Peak-Hour Factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97		
Hourly Flow Rate, HFR	0	487	5	4	784	0		
Percent Heavy Vehicles	0	-	-	0	-	-		
Median Type	Undivided							
RT Channelized			0			0		
Lanes	0	1	0	1	1	0		
Configuration			TR	L	T			
Upstream Signal		1			1			
Minor Street	Westbound			Eastbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume	3	0	3	0	0	0		
Peak-Hour Factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97		
Hourly Flow Rate, HFR	3	0	3	0	0	0		
Percent Heavy Vehicles	0	0	0	0	0	0		
Percent Grade (%)	0			0				
Flared Approach		Y			N			
Storage		1			0			
RT Channelized			0			0		
Lanes	0	1	0	0	0	0		
Configuration		LTR						
Delay, Queue Length, and Level of Service								
Approach	NB	SB	Westbound			Eastbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration		L		LTR				
v (vph)		4		6				
C (m) (vph)		1089		1262				
v/c		0.00		0.00				
95% queue length		0.01		0.01				
Control Delay		8.3		7.9				
LOS		A		A				
Approach Delay	-	-		7.9 ^v				
Approach LOS	-	-		A				

Perris Boulevard/Manzanita Avenue
Intersection LOS Analysis

CAPACITY AND LOS WORKSHEET												
General Information												
Project Description <i>Perris & Manzanita (BO2050 PM) - [6 lanes total]</i>												
Capacity Analysis												
	EB			WB			NB			SB		
Lane group	L	TR		L	TR		L	TR		L	TR	
Adj. flow rate	390	108		24	14		62	961		7	1385	
Satflow rate	1770	1664		1770	1843		1770	5066		1770	4970	
Lost time	2.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	
Green ratio	0.18	0.22		0.18	0.22		0.18	0.26		0.18	0.26	
Lane group cap.	325	361		325	399		325	1309		325	1284	
v/c ratio	1.20	0.30		0.07	0.04		0.19	0.73		0.02	1.08	
Flow ratio	0.22	0.06		0.01	0.01		0.04	0.19		0.00	0.28	
Crit. lane group	Y	Y		N	N		Y	N		N	Y	
Sum flow ratios	0.60											
Lost time/cycle	19.00											
Critical v/c ratio	0.71											
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
Lane group	L	TR		L	TR		L	TR		L	TR	
Adj. flow rate	390	108		24	14		62	961		7	1385	
Lane group cap.	325	361		325	399		325	1309		325	1284	
v/c ratio	1.20	0.30		0.07	0.04		0.19	0.73		0.02	1.08	
Green ratio	0.18	0.22		0.18	0.22		0.18	0.26		0.18	0.26	
Unif. delay d1	49.0	39.4		40.6	37.1		41.5	40.7		40.2	44.5	
Delay factor k	0.50	0.50		0.50	0.50		0.50	0.50		0.50	0.50	
Increm. delay d2	115.8	2.1		0.4	0.2		1.3	3.7		0.1	49.2	
PF factor	1.000	1.000		1.000	1.000		1.000	1.000		1.000	1.000	
Control delay	164.8	41.5		41.0	37.3		42.8	44.4		40.3	93.7	
Lane group LOS	F	D		D	D		D	D		D	F	
Apprch. delay	138.1			39.6			44.3			93.5		
Approach LOS	F			D			D			F		
Intersec. delay	83.3			Intersection LOS						F		

CAPACITY AND LOS WORKSHEET												
General Information												
Project Description <i>Perris & Manzanita (IY2030 PM) - [6 lanes total]</i>												
Capacity Analysis												
	EB			WB			NB			SB		
Lane group	L	TR		L	TR		L	TR		L	TR	
Adj. flow rate	225	104		23	14		59	738		7	1114	
Satflow rate	1770	1664		1770	1843		1770	5062		1770	4996	
Lost time	2.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	
Green ratio	0.18	0.22		0.18	0.22		0.18	0.26		0.18	0.26	
Lane group cap.	325	361		325	399		325	1308		325	1291	
v/c ratio	0.69	0.29		0.07	0.04		0.18	0.56		0.02	0.86	
Flow ratio	0.13	0.06		0.01	0.01		0.03	0.15		0.00	0.22	
Crit. lane group	Y	Y		N	N		Y	N		N	Y	
Sum flow ratios	0.45											
Lost time/cycle	19.00											
Critical v/c ratio	0.53											
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
Lane group	L	TR		L	TR		L	TR		L	TR	
Adj. flow rate	225	104		23	14		59	738		7	1114	
Lane group cap.	325	361		325	399		325	1308		325	1291	
v/c ratio	0.69	0.29		0.07	0.04		0.18	0.56		0.02	0.86	
Green ratio	0.18	0.22		0.18	0.22		0.18	0.26		0.18	0.26	
Unif. delay d1	45.8	39.3		40.5	37.1		41.4	38.6		40.2	42.5	
Delay factor k	0.50	0.50		0.50	0.50		0.50	0.50		0.50	0.50	
Increm. delay d2	11.5	2.0		0.4	0.2		1.2	1.8		0.1	7.8	
PF factor	1.000	1.000		1.000	1.000		1.000	1.000		1.000	1.000	
Control delay	57.3	41.3		41.0	37.3		42.6	40.4		40.3	50.3	
Lane group LOS	E	D		D	D		D	D		D	D	
Aprrch. delay	52.3			39.6			40.6			50.2		
Approach LOS	D			D			D			D		
Intersec. delay	47.0			Intersection LOS						D		

CAPACITY AND LOS WORKSHEET											
General Information											
Project Description <i>Perris & Manzanita (IY2011 PM) - [6 lanes total]</i>											
Capacity Analysis											
Lane group	EB			WB			NB			SB	
	L	TR		L	TR		L	TR		L	TR
Adj. flow rate	58	100		22	13		57	514		7	840
Satflow rate	1770	1664		1770	1841		1770	5053		1770	5039
Lost time	2.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0
Green ratio	0.18	0.22		0.18	0.22		0.18	0.26		0.18	0.26
Lane group cap.	325	361		325	399		325	1305		325	1302
v/c ratio	0.18	0.28		0.07	0.03		0.18	0.39		0.02	0.65
Flow ratio	0.03	0.06		0.01	0.01		0.03	0.10		0.00	0.17
Crit. lane group	Y	Y		N	N		Y	N		N	Y
Sum flow ratios	0.29										
Lost time/cycle	19.00										
Critical v/c ratio	0.35										
Lane Group Capacity, Control Delay, and LOS Determination											
Lane group	EB			WB			NB			SB	
	L	TR		L	TR		L	TR		L	TR
Adj. flow rate	58	100		22	13		57	514		7	840
Lane group cap.	325	361		325	399		325	1305		325	1302
v/c ratio	0.18	0.28		0.07	0.03		0.18	0.39		0.02	0.65
Green ratio	0.18	0.22		0.18	0.22		0.18	0.26		0.18	0.26
Unif. delay d1	41.4	39.2		40.5	37.1		41.3	36.7		40.2	39.6
Delay factor k	0.50	0.50		0.50	0.50		0.50	0.50		0.50	0.50
Increm. delay d2	1.2	1.9		0.4	0.2		1.2	0.9		0.1	2.5
PF factor	1.000	1.000		1.000	1.000		1.000	1.000		1.000	1.000
Control delay	42.6	41.1		40.9	37.2		42.5	37.6		40.3	42.1
Lane group LOS	D	D		D	D		D	D		D	D
Apprch. delay	41.6			39.6			38.1			42.1	
Approach LOS	D			D			D			D	
Intersec. delay	40.6			Intersection LOS						D	

CAPACITY AND LOS WORKSHEET												
General Information												
Project Description <i>Perris Blvd Street Widening - Perris & Manzanita (BO2050 PM)</i>												
Capacity Analysis												
	EB			WB			NB			SB		
Lane group	L	TR		L	TR		L	TR		L	TR	
Adj. flow rate	390	108		24	14		62	961		7	1385	
Satflow rate	1770	1664		1770	1843		1770	3526		1770	3459	
Lost time	2.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	
Green ratio	0.18	0.22		0.18	0.22		0.18	0.26		0.18	0.26	
Lane group cap.	325	361		325	399		325	911		325	894	
v/c ratio	1.20	0.30		0.07	0.04		0.19	1.05		0.02	1.55	
Flow ratio	0.22	0.06		0.01	0.01		0.04	0.27		0.00	0.40	
Crit. lane group	Y	Y		N	N		Y	N		N	Y	
Sum flow ratios	0.72											
Lost time/cycle	19.00											
Critical v/c ratio	0.86											
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
Lane group	L	TR		L	TR		L	TR		L	TR	
Adj. flow rate	390	108		24	14		62	961		7	1385	
Lane group cap.	325	361		325	399		325	911		325	894	
v/c ratio	1.20	0.30		0.07	0.04		0.19	1.05		0.02	1.55	
Green ratio	0.18	0.22		0.18	0.22		0.18	0.26		0.18	0.26	
Unif. delay d1	49.0	39.4		40.6	37.1		41.5	44.5		40.2	44.5	
Delay factor k	0.50	0.50		0.50	0.50		0.50	0.50		0.50	0.50	
Increm. delay d2	115.8	2.1		0.4	0.2		1.3	45.4		0.1	252.7	
PF factor	1.000	1.000		1.000	1.000		1.000	1.000		1.000	1.000	
Control delay	164.8	41.5		41.0	37.3		42.8	89.9		40.3	297.2	
Lane group LOS	F	D		D	D		D	F		D	F	
Aprch. delay	138.1			39.6			87.0			295.9		
Approach LOS	F			D			F			F		
Intersec. delay	193.6			Intersection LOS						F		

CAPACITY AND LOS WORKSHEET												
General Information												
Project Description <i>Perris Blvd Street Widening - Perris & Manzanita (IY2030 PM)</i>												
Capacity Analysis												
	EB			WB			NB			SB		
Lane group	L	TR		L	TR		L	TR		L	TR	
Adj. flow rate	225	104		23	14		59	738		7	1114	
Satflow rate	1770	1664		1770	1843		1770	3523		1770	3477	
Lost time	2.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	
Green ratio	0.18	0.22		0.18	0.22		0.18	0.26		0.18	0.26	
Lane group cap.	325	361		325	399		325	910		325	898	
v/c ratio	0.69	0.29		0.07	0.04		0.18	0.81		0.02	1.24	
Flow ratio	0.13	0.06		0.01	0.01		0.03	0.21		0.00	0.32	
Crit. lane group	Y	Y		N	N		Y	N		N	Y	
Sum flow ratios	0.54											
Lost time/cycle	19.00											
Critical v/c ratio	0.65											
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
Lane group	L	TR		L	TR		L	TR		L	TR	
Adj. flow rate	225	104		23	14		59	738		7	1114	
Lane group cap.	325	361		325	399		325	910		325	898	
v/c ratio	0.69	0.29		0.07	0.04		0.18	0.81		0.02	1.24	
Green ratio	0.18	0.22		0.18	0.22		0.18	0.26		0.18	0.26	
Unif. delay d1	45.8	39.3		40.5	37.1		41.4	41.8		40.2	44.5	
Delay factor k	0.50	0.50		0.50	0.50		0.50	0.50		0.50	0.50	
Increm. delay d2	11.5	2.0		0.4	0.2		1.2	7.8		0.1	117.7	
PF factor	1.000	1.000		1.000	1.000		1.000	1.000		1.000	1.000	
Control delay	57.3	41.3		41.0	37.3		42.6	49.5		40.3	162.2	
Lane group LOS	E	D		D	D		D	D		D	F	
Apprch. delay	52.3			39.6			49.0			161.5		
Approach LOS	D			D			D			F		
Intersec. delay	104.5			Intersection LOS						F		

CAPACITY AND LOS WORKSHEET												
General Information												
Project Description <i>Perris Blvd Street Widening - Perris & Manzanita (IY2011 PM)</i>												
Capacity Analysis												
	EB			WB			NB			SB		
Lane group	L	TR		L	TR		L	TR		L	TR	
Adj. flow rate	58	100		22	13		57	514		7	840	
Satflow rate	1770	1664		1770	1841		1770	3516		1770	3507	
Lost time	2.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	
Green ratio	0.18	0.22		0.18	0.22		0.18	0.26		0.18	0.26	
Lane group cap.	325	361		325	399		325	908		325	906	
v/c ratio	0.18	0.28		0.07	0.03		0.18	0.57		0.02	0.93	
Flow ratio	0.03	0.06		0.01	0.01		0.03	0.15		0.00	0.24	
Crit. lane group	Y	Y		N	N		Y	N		N	Y	
Sum flow ratios	0.36											
Lost time/cycle	19.00											
Critical v/c ratio	0.43											
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
Lane group	L	TR		L	TR		L	TR		L	TR	
Adj. flow rate	58	100		22	13		57	514		7	840	
Lane group cap.	325	361		325	399		325	908		325	906	
v/c ratio	0.18	0.28		0.07	0.03		0.18	0.57		0.02	0.93	
Green ratio	0.18	0.22		0.18	0.22		0.18	0.26		0.18	0.26	
Unif. delay d1	41.4	39.2		40.5	37.1		41.3	38.7		40.2	43.4	
Delay factor k	0.50	0.50		0.50	0.50		0.50	0.50		0.50	0.50	
Increm. delay d2	1.2	1.9		0.4	0.2		1.2	2.6		0.1	16.7	
PF factor	1.000	1.000		1.000	1.000		1.000	1.000		1.000	1.000	
Control delay	42.6	41.1		40.9	37.2		42.5	41.2		40.3	60.1	
Lane group LOS	D	D		D	D		D	D		D	E	
Apprch. delay	41.6			39.6			41.3			60.0		
Approach LOS	D			D			D			E		
Intersec. delay	51.1			Intersection LOS						D		

CAPACITY AND LOS WORKSHEET												
General Information												
Project Description <i>Perris Blvd Street Widening - Perris & Manzanita (EX PM)</i>												
Capacity Analysis												
	EB			WB			NB			SB		
Lane group	L	TR		L	TR		L	TR		L	TR	
Adj. flow rate	23	99		22	13		56	467		7	782	
Satflow rate	1770	1665		1770	1841		1770	3514		1770	3516	
Lost time	2.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	
Green ratio	0.18	0.22		0.18	0.22		0.18	0.26		0.18	0.26	
Lane group cap.	325	361		325	399		325	908		325	908	
v/c ratio	0.07	0.27		0.07	0.03		0.17	0.51		0.02	0.86	
Flow ratio	0.01	0.06		0.01	0.01		0.03	0.13		0.00	0.22	
Crit. lane group	Y	Y		N	N		Y	N		N	Y	
Sum flow ratios	0.33											
Lost time/cycle	19.00											
Critical v/c ratio	0.39											
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
Lane group	L	TR		L	TR		L	TR		L	TR	
Adj. flow rate	23	99		22	13		56	467		7	782	
Lane group cap.	325	361		325	399		325	908		325	908	
v/c ratio	0.07	0.27		0.07	0.03		0.17	0.51		0.02	0.86	
Green ratio	0.18	0.22		0.18	0.22		0.18	0.26		0.18	0.26	
Unif. delay d1	40.5	39.1		40.5	37.1		41.3	38.1		40.2	42.4	
Delay factor k	0.50	0.50		0.50	0.50		0.50	0.50		0.50	0.50	
Increm. delay d2	0.4	1.9		0.4	0.2		1.1	2.1		0.1	10.5	
PF factor	1.000	1.000		1.000	1.000		1.000	1.000		1.000	1.000	
Control delay	41.0	41.0		40.9	37.2		42.5	40.1		40.3	53.0	
Lane group LOS	D	D		D	D		D	D		D	D	
Apprch. delay	41.0			39.6			40.4			52.9		
Approach LOS	D			D			D			D		
Intersec. delay	47.1			Intersection LOS						D		